Buoyancy & Pipe Flotation

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Buoyancy & Pipe Flotation

- Why is this a timely topic?
- What is buoyancy?
- TWO methods to account for this our designs
- Factors of Safety
- Where the rubber meets the road...example problems!















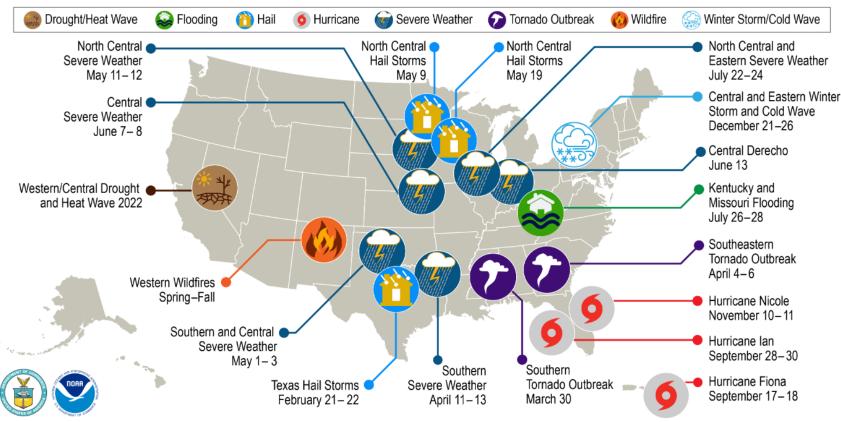




Changing Weather Patterns

Costly Disasters

U.S. 2022 Billion-Dollar Weather and Climate Disasters



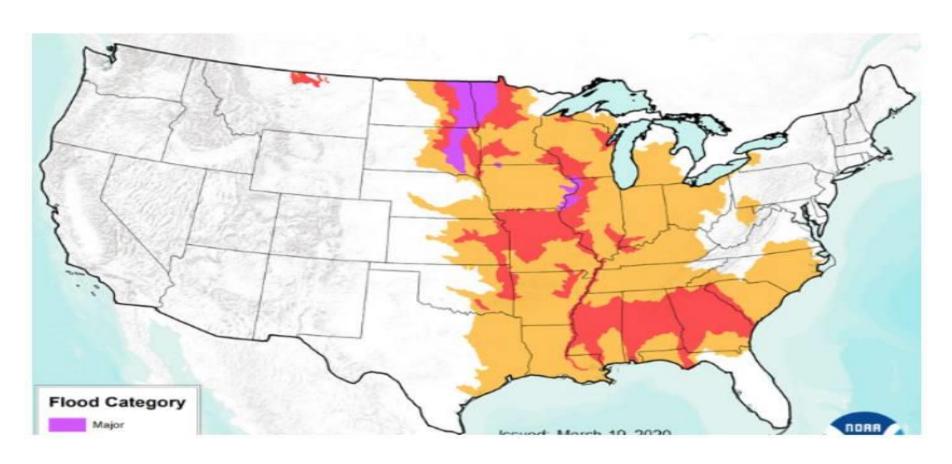
This map denotes the approximate location for each of the 18 separate billion-dollar weather and climate disasters that impacted the United States in 2022.





Changing Weather Patterns

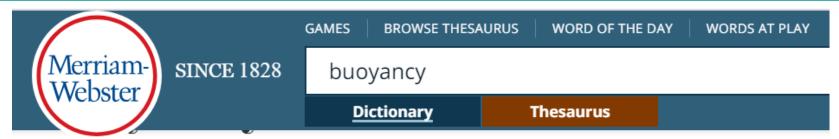
Prolonged Flooding







Buoyancy





buoy·an·cy | \ 'boi-ən(t)-sē →, 'bü-yən(t)- \

Definition of *buoyancy*

- 1 a : the tendency of a body to float or to rise when submerged in a fluid // testing an object's buoyancy
 - **b** chemistry: the power of a fluid to exert an upward force on a body placed in it

// the *buoyancy* of water

also: the upward force exerted





Buoyancy

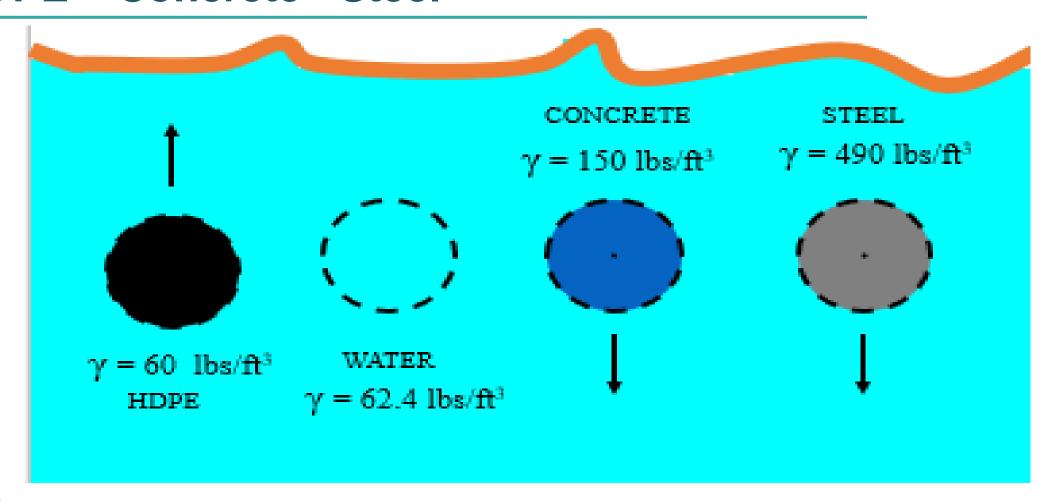






Relative Pipe Weights

HDPE – Concrete - Steel



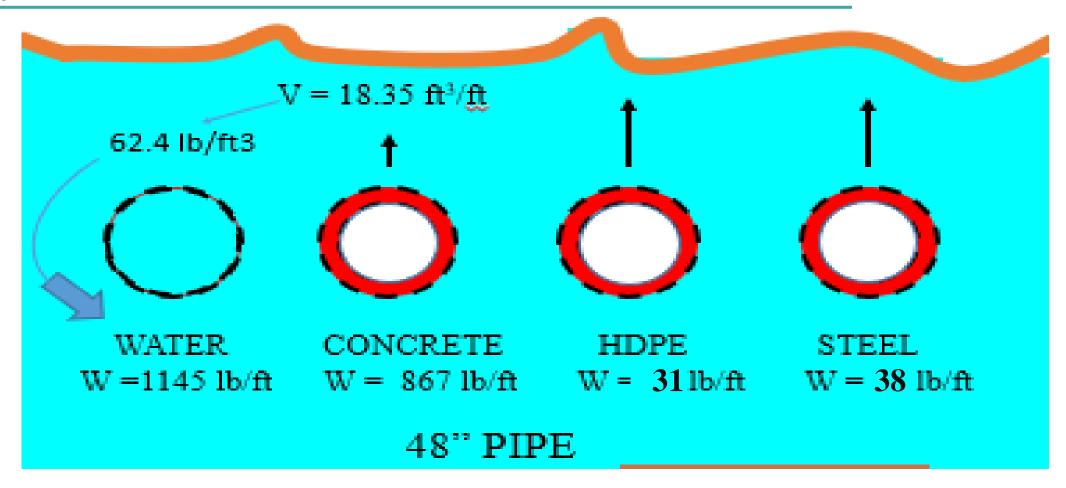






Pipe Weights

Why does this matter?







Buried Pipe

When to be concerned?





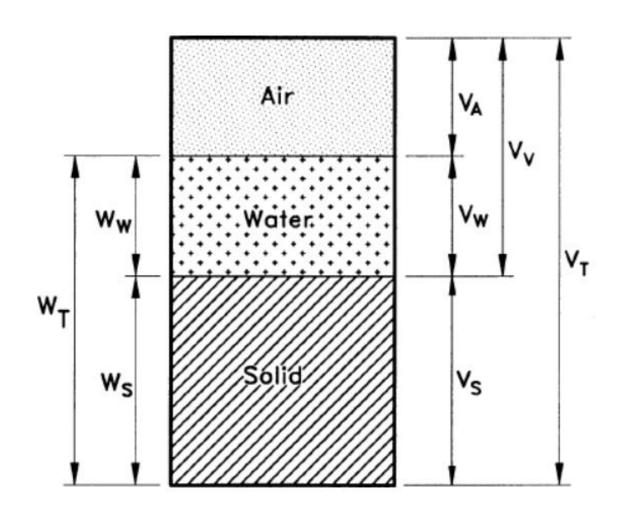


 $\gamma_t = 120 \text{ lbs/ft}^3$





Microstructure of Soil





Buoyant Weight of Soil

$$\gamma_b = \gamma_t - \gamma_w$$

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\gamma_t = saturated unit weight of soil (pcf)
\gamma_w = unit weight of water (pcf)
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Calculations

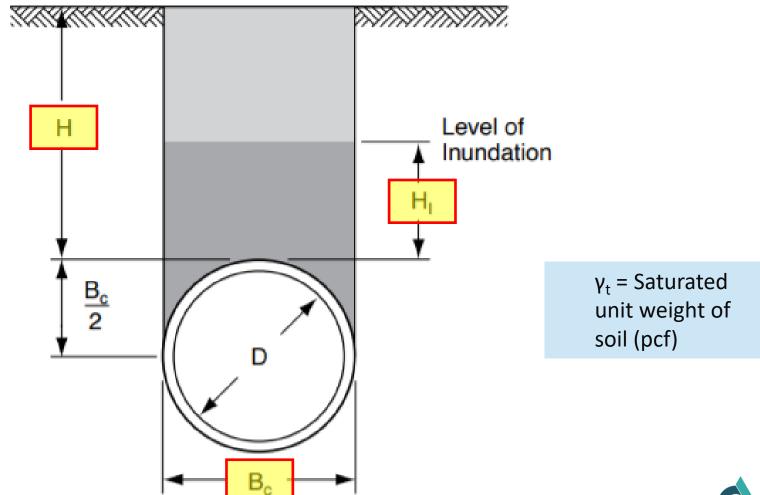






Required Information

What do we need to know?







Design Data 22

ACPA Document

www.concretepipe.org

Resources
Design
Design Data



ACPA » Design Data 22

Flotation of Circular Concrete Pipe

There are several installation conditions where there is the possibility that concrete pipe may float, even though the density of concrete is approximately 2.4 times that of water. Some of these conditions are: the use of flooding to consolidate backfill; pipelines in areas which will be inundated, such as a flood plain or under a future man-made lake; subaqueous pipelines; flowable fill installations; and pipelines in areas with a high groundwater table. When such conditions exist, floation probability should be checked.

Flotation Factors

The buoyancy of concrete pipe depends upon the weight of the pipe, the weight of the volume of water deplaced by the pipe, the weight of the ligid load carried by the pipe and the weight of the backfil. As a conservative practice in analysis, the line should be considered empty so the weight of any future liquid load is then an additional safely factor.

Design Procedure

- A suggested seven step logical procedure is presented for determining the degree of buoyancy of an empty concrete pipeline and possible measures needed to prevent fotation. Downward forces are considered positive and upward forces are considered negative.
- Determine the downward force of the pipe weight in pounds per linear foot of pipe.
- 2. Determine the bouyant upward force of the weight of the displaced water in pounds per linear foot of pipe.
 3. Find the algebraic sum of the force determined in
 Steps 1 and 2. If the resultant force is positive, the pipe will not float. If the resultant force is negative, proceed with Step 4.
- Determine the downward force of the total weight of backfill in pounds per linear foot of pipe.
 Apply a factor of safety to determine the decreased.
- total weight of backfil.

 6. Find the algebraic sum of the downward force
- determined in Step 5 and the excess upward force determined in Step 3. If the resultant force is positive, the pipe will not float. If the resultant force is negative,
- Select and analyze the procedures to be used to prevent flotation.

Preventive Procedures

If the total weight of the pipe and backfill is not adequate to prevent flotation of the pipe, preventative non-flotation procedures, such as additional backfill, mechanical anchorage, heavier pipes, or combinations of these would be required. Some of the commonly used methods are:

- 1. Increased wall thickness.
- 2. Precast or cast-in-place concrete collars.
- Precast or cast-in-place concrete blocks, fastened by suitable means.
- 4. Pipe strapped to piles or concrete anchor slab
- 5. Additional backfill.

When computing the volume of concrete required per linear foot for pipe anchorage, remember that concrete, which weighs 150 pounds per cubic foot in air, weighs only 87.6 pounds per cubic foot under water.

Factor of Safety

Construction soils are noted for tack of unsformity. Depending on the eatent of information of the proposed backfill material and site condition, a factor of safety ranging between 1.0 and plant of decrease the downward force of the backfill. Cleansity, if the weight of the structure is the primary force resisting flotation than a safety factor of 1.0 is adequate. However, if fishion or cohesion are the primary forces resisting flotation, then a higher safety factor would be more searching flotation, then a higher safety factor would be more

Consideration must also be given to the interface between layers of differing soil types. If fine grained soils (such as clays or alls) are placed adjacent to coarse grained soils (such as crushed stone), upon wetting, those layers may combine at the interface thereby allowing the pipe to fost at distance equal to the loss in volume. Increased factor of safety in combination with layer separation methods are recommended.





Watkins Moser Method

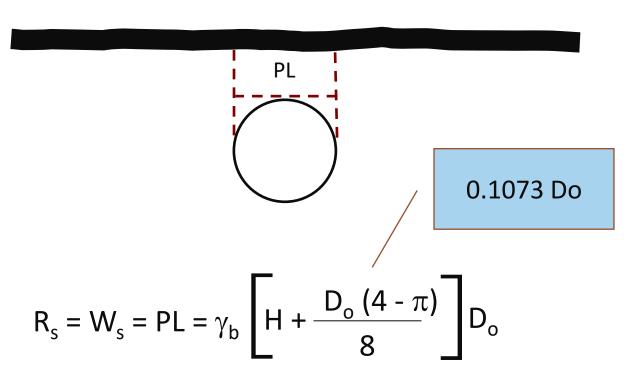


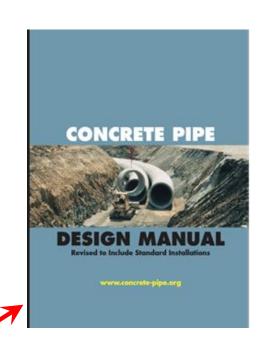


Calculating Soil Resistance

Design Data 22

$$\gamma_b = \gamma_t - \gamma_w$$
 $R_s = Soil Resistance$
 $W_s = Weight of Soil$
 $PL = Prism Load$





Equation 4.2 – Concrete Pipe Design Manual

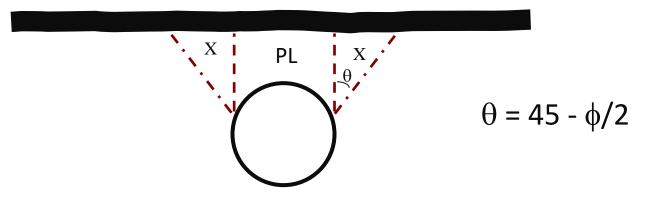




Calculating Soil Resistance

Watkins-Moser Method

$$\gamma_b = \gamma_t - \gamma_w$$
 $R_s = Soil Resistance$
 $W_s = Weight of Soil$
 $PL = Prism Load$



$$R_s = PL + 2 X$$

$$2X = [(H + D_o/2)^2 \tan(45 - \phi/2)]\gamma_b$$

 ϕ = internal angle of friction



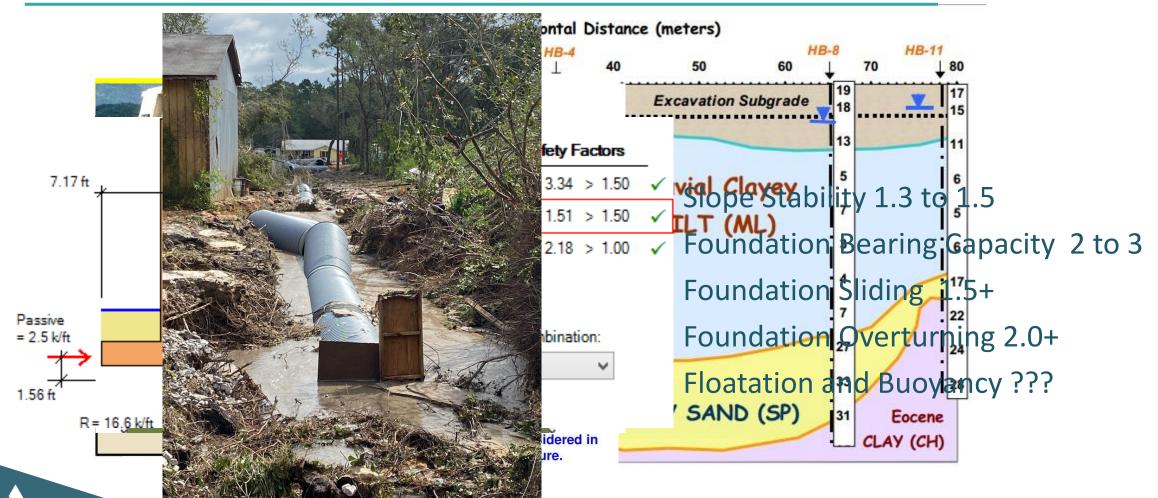








Geotechnical Eng. – LRFD Bridge Structures















Process:

Calculate Buoyant Force
Calculate Soil Resistance
Solve for the Net Force





Reinforced Concrete Pipe

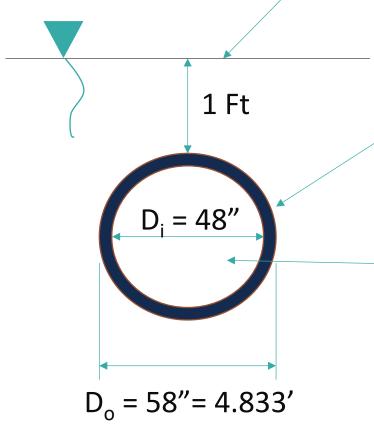
Given:

$$\gamma_t$$
 = 120 pcf

$$\gamma_{\rm w}$$
 = 62.4 pcf

$$\phi$$
 = 30 deg

Existing Ground Surface



RC Pipe Weight
$$W_p = 867 \text{ lbs/ft}$$

Weight of Water Displaced

$$W_w = \pi (d_o/2)^2 \gamma_w$$

 $W_w = \pi (4.833/2)^2 \times 62.4$

$$W_{w} = 1,145 \text{ lbs/ft}$$

$$BF = -278 lbs/ft$$





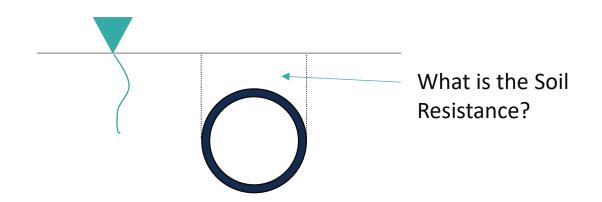
Calculate soil resistance (Rs) – ACPA Method

$$\gamma_b = \gamma_t - \gamma_w$$

 R_s = Soil Resistance

W_s = Weight of Soil

PL = Prism Load



$$R_s = W_s = PL = \gamma_b \left[H + \frac{D_o (4 - \pi)}{8} \right] D_o$$
 (eq. 4.2)

$$R_s = (120 - 62.4) \left[1 + \frac{4.833(4 - \pi)}{8} \right] 4.833$$

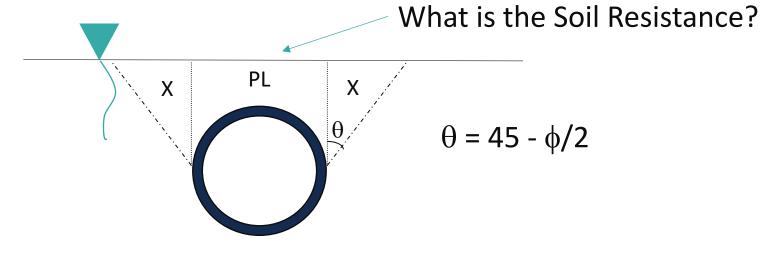




Calculate soil resistance (Rs) – Watkins-Moser Method

Given:

$$\gamma_t$$
 = 120 pcf
 γ_w = 62.4 pcf
 ϕ = 30 deg



$$R_s = PL + 2X$$

$$2X = [(H + D_o/2)^2 \tan(45 - \phi/2)] \gamma_b$$

$$2X = [(1 + 4.833/2)^2 \tan(45 - 30/2)] (120-62.4)$$

$$2X = 388 lbs/ft$$

$$R_s = 423 + 388 = 811 lbs/ft$$





Calculate Net Force

ACPA Method

Net force = BF + (
$$R_s$$
 / FS)
= (-278) + (423/1.25) = 60 lbs/ft

Watkins/Moser Method

Net force = BF +
$$(R_s / FS)$$

= $(-278) + (811/2.0) = 128$ lbs

Both Methods Show That The Pipe Will Not Float.





Results – Reinforced Concrete Pipe

RCP Results

Method	Buoyancy Force, BF (lbs/ft)	Soil Resistance R _s (lbs/ft)	Factor of Safety, FS	Net Force (lbs/ft)
ACPA	-278	423	1.25	60
W/M	-278	811	2.0	128





Calculate the buoyant force (BF) – CMP & HDPE

$$\gamma_t = 120 \text{ pcf}$$

$$\gamma_{\rm w}$$
 = 62.4 pcf

$$\phi$$
 = 30 deg

CMP Pipe Weight

$$W_p = 38 \text{ lb/ft}$$

Weight of Water Displaced

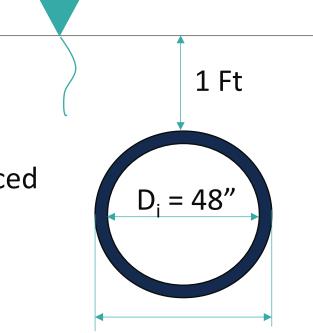
$$W_{w} = \pi \left(d_{o}/2 \right)^{2} \gamma_{w}$$

$$W_w = 957 lbs/ft$$

$$BF = -919 lbs/ft$$

$$R_s = 375 lbs/ft - ACPA$$

$$R_s = 717 lbs/ft - W/M$$



CMP -
$$D_o = 53'' = 4.42'$$

HDPE - $D_o = 54'' = 4.50'$

HDPE Pipe Weight

$$W_p = 31 \text{ lb/ft}$$

Weight of Water Displaced

$$W_{w} = \pi \left(d_{o}/2 \right)^{2} \gamma_{w}$$

$$W_w = 992 lbs/ft$$

$$R_s = 383 lbs/ft - ACPA$$

$$R_s = 734 lbs/ft - W/M$$

Comparison – Different Pipe Materials

Comparison – ACPA Method

48" Pipe Type	Buoyancy Force, BF (lbs/ft)	Soil Resistance R _s (lbs/ft)	Factor of Safety, FS	Net Force (lbs/ft)
СМР	-919	375	1.25	-619
HDPE	-961	383	1.25	-654
RCP	-278	423	1.25	60





Comparison – Different Pipe Materials

How Much Fill For a 48 Inch Pipe?

	Pipe Type			
Method	RCP	CMP	HDPE	
ACPA	9"	49"	50"	
M/W	6"	34"	36"	





^{*}For plastic pipe, a good rule of thumb is fill height equal to pipe diameter.

Example Standards

How is Floatation Addressed?



Designation: D2321 - 20

ASTM D2321 – Section 7.6 Minimum Cover

	um Co Class To precl		_
1 2 listurbanc	e to pipezarnbedmen	t, a mini m ım depth	of backfi48"
18 to 1	pipe should be main onstruction equipme	tained before allowing	ng vehicles,
2 ⁴ The minin	num depth of cover s	should be established	trench. 1 by the 48"
	ased on an evaluation		
36 In the absorption	ence of an engineering	ng evaluation, the fol	lowing 48"
42" materials	cover requirements and accordanged in accordanged i	should be used. For a ce with Table 3, prov	embedment 48" 7ide cover
	pth of bæ¶rfill above		
Ŭ ĥ	one pipodiameter (v	J .	
60, embedmer 60, diameter (nt, and a coyer of at 1 60 whichever is larger)	least 36 in. (0.9 m) o 60 for Class II, III, and	r one pipe 48"



Example

Reinforced Concrete Can Float





Resist the Float

Minimum Cover for Concrete Pipe* – ACPA Method

Pipe Size	Min. Fill	Pipe Size	Min. Fill	Pipe Size	Min. Fill
21"	1"	42"	7"	78"	18"
24"	2"	48"	9"	84"	21"
27"	2"	54"	10"	90"	23"
30"	3"	60"	13"	96"	24"
33"	4"	66"	15"	102"	27"
36"	5"	72"	17"	108"	29"

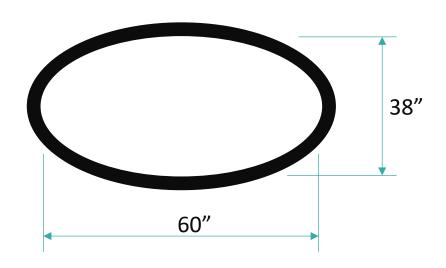


^{*}Assumes the groundwater table is at the surface.

Horizontal Elliptical Pipe – 48" Equivalent (38" x 60")

Illustration 5.3 Dimensions and Approximate Weights of Elliptical Concrete Pipe

ASTM C 507-Reinforced Concrete Elliptical Culvert, Storm Drain and Sewer Pipe							
Equivalent Round Size, inches	Minor Axis, inches	Major Axis, inches	Minimum Wall Thickness, inches	Water-Way Area, square feet	Approximate Weight, pounds per foot		
18	14	23	2 3/4	1.8	195		
24	19	30	3 1/4	3.3	300		
27	22	34	3 1/2	4.1	365		
30	24	38	3 3/4	5.1	430		
33	27	42	3 3/4	6.3	475		
36	29	45	4 1/2	7.4	625		
39	32	49	4 3/4	8.8	720		
42	34	53	5	10.2	815		
48	38	60	5 1/2	12.9	1000		
54	43	68	6	16.6	1235		
60	48	76	6 1/2	20.5	1475		
66	53	83	7	24.8	1745		
72	58	91	7 1/2	29.5	2040		







Horizontal Elliptical Pipe – 48" Equivalent (38" x 60")

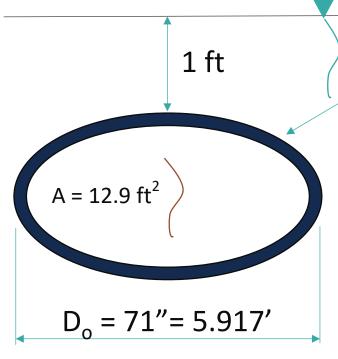
Given:

Ground water level at the surface:

$$\gamma_t$$
 = 120 pcf

$$\gamma_{\rm w}$$
 = 62.4 pcf

 ϕ = 30 deg



RC Pipe Weight $W_n = 1000 \text{ lb/ft}$

Area of Water Displaced A = 19.64 ft²

Weight of Water Displaced $W_w = 1,226 lbs/ft$

BF = - 226 lbs/ft

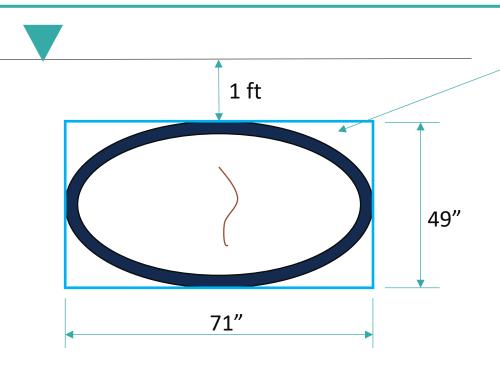




Horizontal Elliptical Pipe – 48" Equivalent (38" x 60")

Rise = 38" Span = 60"

Wall = 5.5"



What is the Soil Resistance from the Upper Haunches?

Total Rectangle Area $(49 \times 71)/144 = 24.16 \text{ ft}^2$

Pipe Area $A = 19.64 \text{ ft}^2$

Upper Haunch Area $A = (24.16 - 19.64)/2 = 2.26 \text{ ft}^2$

Upper Haunch Soil Resistance $R_s = 2.26 \times (120-62.4) = 130 \text{ lbs/ft}$



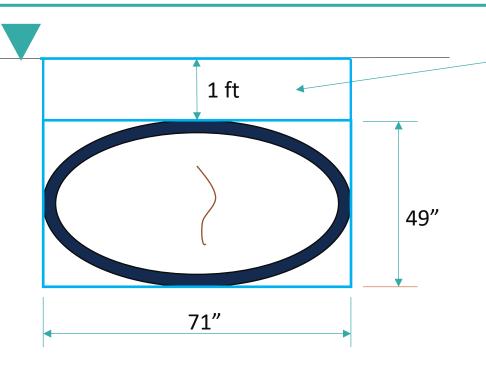


Horizontal Elliptical Pipe – 48" Equivalent (38" x 60")

Rise = 38"

Span = 60"

Wall = 5.5''



What is the Soil Resistance from the Soil Prism Above the Crown?

Upper Rectangle Area $(12 \times 71)/144 = 5.92 \text{ ft}^2$

Upper Rectangle Soil Resistance $R_s = 5.92 \times (120-62.4) = 341 \text{ lbs/ft}$

Total Soil Resistance

Rs = Upper Haunch + Rectangular Soil Prism

$$R_s = 130 + 341 = 471 lbs/ft$$





Comparison – Different Pipe Shapes – 48"

Shape	Buoyancy Force, BF (lbs/ft)	Soil Resistance R _s (lbs/ft)	Factor of Safety, FS	Net Force (lbs/ft)
Elliptical	-226	471	1.25	150
Circular	-278	423	1.25	60







HDPE Pipe when Water Table Not at the Surface

Given:

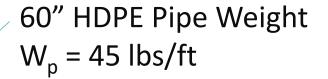
60" HDPE

 $\gamma_t = 130 \text{ pcf}$

 $\gamma_d = 110 \text{ pcf}$

 $\gamma_{\rm w}$ = 62.4 pcf





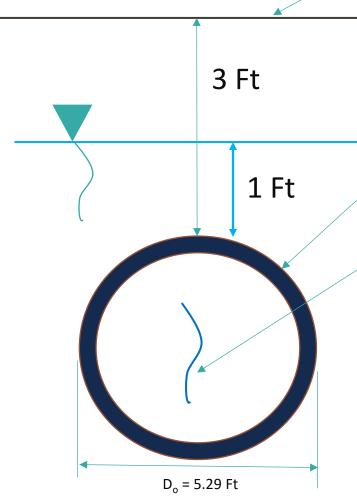
Weight of Water Displaced

$$W_{w} = \pi (d_{o}/2)^{2} \gamma_{w}$$

 $W_{w} = 1373 \text{ lbs/ft}$







HDPE Pipe when Water Table Not at the Surface

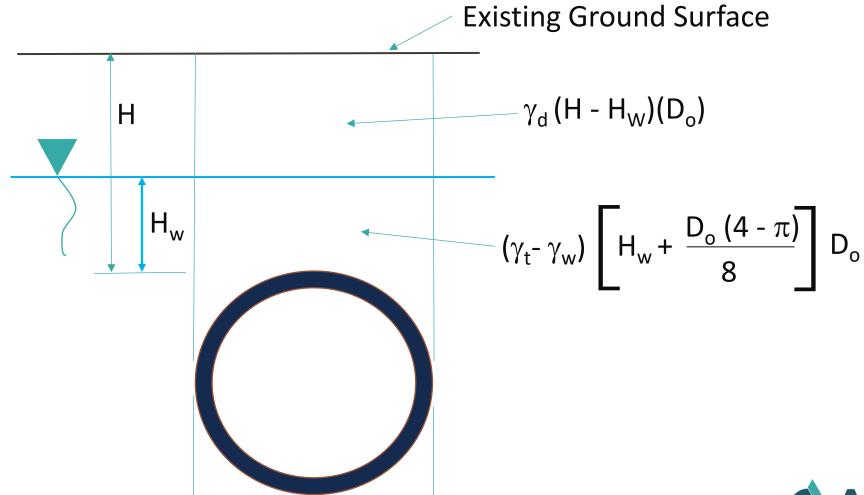
Given:

60" HDPE

 γ_{t} = 130 pcf

 $\gamma_d = 110 \text{ pcf}$

 $\gamma_{\rm w}$ = 62.4 pcf



 $D_0 = 5.29 \text{ Ft}$



HDPE Pipe when Water Table Not at the Surface

Calculation of Soil Resistance

$$R_s = (\gamma_t - \gamma_w) \left[H_w + \frac{D_o (4 - \pi)}{8} \right] D_o + \gamma_d (H - H_w)(D_o)$$

$$R_s = (130 - 62.4) \left[1.0 + \frac{5.29 (4 - \pi)}{8} \right] 5.29 + 110 (3-1.0)(5.29)$$

$$R_s = 561 + 1163$$

$$R_s = 1724 lbs/ft$$





HDPE Pipe when Water Table Not at the Surface

60" HDPE Results – 3 Ft. Cover

Water	Buoyancy	Soil	Factor of	Net
Over	Force, BF	Resistance	Safety, FS	Force
Pipe (ft.)	(lbs/ft)	R _s (lbs/ft)		(lbs/ft)
1.0	-1328	1724	1.25	51
1.0	-1320	1/24	1.23	ЭŢ





HDPE Pipe when Water Table Not at the Surface

3 Ft

 $D_0 = 5.29 \text{ Ft}$

1.5 Ft

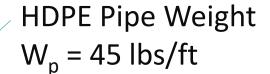
Given:

 $\gamma_t = 130 \text{ pcf}$

 $\gamma_d = 110 \text{ pcf}$

 $\gamma_{\rm w}$ = 62.4 pcf





Weight of Water Displaced

$$W_{w} = \pi (d_{o}/2)^{2} \gamma_{w}$$

$$W_w = 1373 lbs/ft$$





HDPE Pipe when Water Table Not at the Surface

Calculation of Soil Resistance

$$R_s = (\gamma_t - \gamma_w) \left[H_w + \frac{D_o (4 - \pi)}{8} \right] D_o + \gamma_d (H - H_w)(D_o)$$

$$R_s = (130 - 62.4)$$
 $\left[\frac{1.5}{8} + \frac{5.29 (4 - \pi)}{8} \right] 5.29 + 110 (3-1.5)(5.29)$

$$R_s = 739 + 873$$

$$R_{s} = 1612 lbs/ft$$





HDPE Pipe when Water Table Not at the Surface

60" HDPE Results – 3 Ft. Cover

Water Over Pipe (ft.)	Buoyancy Force, BF (lbs/ft)	Soil Resistance R _s (lbs/ft)	Factor of Safety, FS	Net Force (lbs/ft)
1.0	-1328	1724	1.25	51
1.5	-1328	1612	1.25	-38







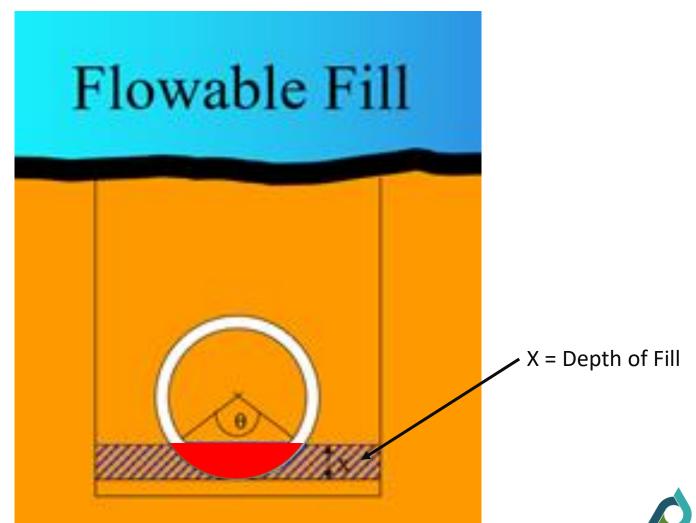
Flowable Fill

When is the pipe subject to floatation?

 $\gamma_{\rm ff}$ = 130 pcf

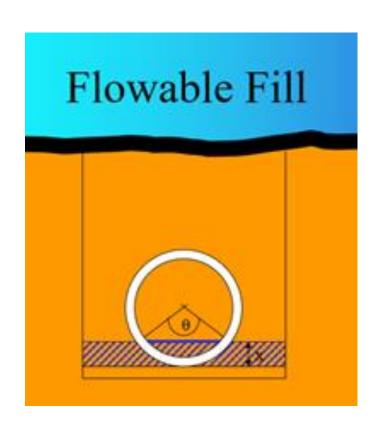
Area of Segment of Circle

Calculate Weight of Displaced Fill





Flowable Fill - Controlled Low Strength Material - CLSM

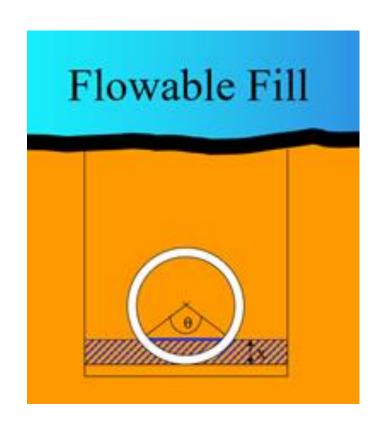


HDPE	Pipe								
Pipe	Pipe	Weight of	Density of		Area of	Wt of CLSM	Weight of	Bouyant	
Size	OD (ft)	Pipe	CLSM	Height of CLSM	Segment	Displaced	Pipe	Force	% of Do
12	1.21	3.2	130	2	13.75	-12.41	3.2	-9.21	13.80%
15	1.50	4.6	130	2	15.46	-13.95	4.6	-9.35	11.11%
18	1.83	6.4	130	2	17.20	-15.52	6.4	-9.12	9.09%
24	2.33	11	130	2	19.51	-17.61	11	-6.61	7.15%
30	3.00	15.4	130	2	22.25	-20.08	15.4	-4.68	5.56%
36	3.50	19.8	130	3	43.92	-39.65	19.8	-19.85	7.14%
42	4.00	26.4	130	3	47.09	-42.51	26.4	-16.11	6.25%
48	4.50	31.3	130	3	50.05	-45.19	31.3	-13.89	5.56%
60	5.58	45.2	130	3	55.94	-50.50	45.2	-5.30	4.48%





Flowable Fill - Controlled Low Strength Material - CLSM



Wt of CLSM Displaced -105.14	Weight of Pipe 93	Bouyant Force -12.14	% of Do
Displaced -105.14	Pipe	Force	
-105.14	-		
	93	-12.14	
120 21			56.26%
-139.21	127	-12.21	51.28%
-177.16	168	-9.16	47.83%
-265.06	264	-1.06	43.33%
-402.08	384	-18.08	43.24%
-528.34	524	-4.34	40.91%
-716.00	686	-30.00	41.18%
-880.69	867	-13.69	39.66%
-1322.14	1295	-27.14	38.89%
	-265.06 -402.08 -528.34 -716.00 -880.69	-177.16 168 -265.06 264 -402.08 384 -528.34 524 -716.00 686 -880.69 867	-177.16 168 -9.16 -265.06 264 -1.06 -402.08 384 -18.08 -528.34 524 -4.34 -716.00 686 -30.00 -880.69 867 -13.69







































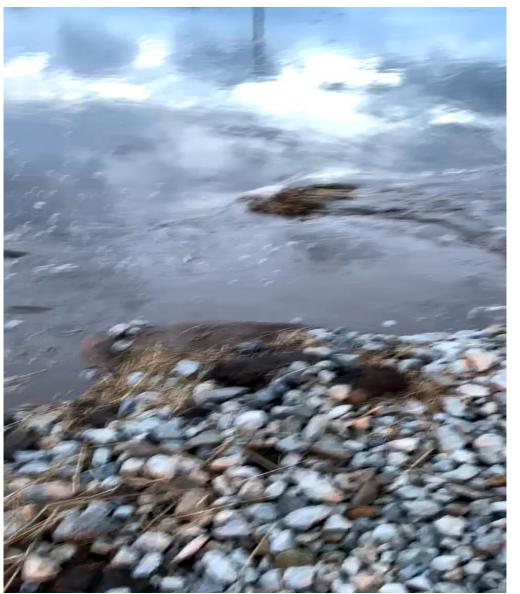






















The Pyramids of Egypt

HOW WERE THEY REALLY

Buoyancy& Pipe Flotation





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